Vibration Analysis of Simply Supported Rectangular Plates Constrained by Rotational Edge Springs

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Abstract

Comprehensive and accurate numerical results are presented for natural frequencies of thin isotropic, simply supported rectangular plates additionally constrained by rotational elastic springs on the edges. For complete coverage of combination, the number of all the combinations in boundary conditions of simply supported, rotationally constrained and clamped is calculated by Polya counting theory, and all sets of frequency parameters are tabulated for the lowest five modes. The Ritz method, along with displacements assumed in special polynomial form with boundary index, is used to include the strain energy stored in the rotational springs. Convergence and comparison studies are made to demonstrate accuracy of tabulated results, and the frequency parameters are listed for the fifteen sets of boundary conditions and various spring stiffness of the square plates. Some results are also presented for rectangular plates.

Keywords: Free vibration, natural frequency, simply supported plate, rectangular plate, rotational spring

1. Introduction

Vibration of flat plates has been an important research topic in mechanical, aeronautical and other structureoriented fields in engineering. A large number of publications has appeared since a monograph was compiled by Leissa [1] in 1969, and some good textbooks were released, for example a book written by Gorman [2] on plate vibration. Among various plate applications, the most typical planform is a rectangle, and Leissa [3] published a paper on frequency parameters to cover all combinations of free, simply supported and clamped edges. These results are updated by the present author in improved accuracy [4].

For rectangular plates elastically constrained along edges, a reasonable number of papers were published because of the importance that plate structural elements are usually attached elastically to the main structure. Up to the year of 2000, Laura and his co-workers published some papers [5-8] to obtain lower frequencies, and other authors [9,10] dealt with plates with springs. A series of notable works were written by Gorman [11-15] on vibration of rectangular plates with elastic edge supports by using a famous method of the superposition method.

In the 2000's, Li [16] and Li [17] presented solutions for predicting frequencies of rectangular plates with generally restrained edges. Eftekhari and Jafari [18] derived a solution for variable thickness plates with elastic edges. Recently, Wan [19] presented an original analysis on the topic, and Zhang and others [20] presented in 2021 some results on plates with free and opposite two adjacent

elastic edges. Leng and others [21] studied in 2022 vibration of plates with one corner free and its edges rotationally-restrained. Thus, up to now, the vibration of rectangular plates with elastic edge springs has drawn attentions from researchers, but the purpose of all the previous papers seems to propose analytical methods, and the frequency data presented are still limited to some specific cases.

 The present author therefore undertakes to compile comprehensive and organized sets of frequency parameters for the problem, and published already one paper [22] on the free plates elastically supported by translational springs. In this paper, an analysis is extended to simply supported rectangular plates with rotational spring on the edges. These two papers fully encompass elastically supported rectangular plates in a way to cover

Figure 1. Simply supported rectangular plate with uniform rotational springs on the edges and the coordinate system

from a totally free to a simply supported plate [22] and from there to a totally clamped plate (present paper).

2. Methodology

2.1. Combination of boundary conditions

 Figure 1 illustrates an isotropic rectangular plate simply supported along four edges and elastically constrained by additional uniform rotational spring at the edges, and this edge condition is denoted by RS (Rotational Spring) in the paper. The dimension of the plate is given by $a \times b \times h$ (thickness) and the origin of the coordinate system is located in the center. Starting from the left hand edge (*x= -a*/2), four edges are labelled as Edge(1), Edge(2), Edge(3) and Edge(4) in counter-clock-wise, and each spring can have different value of rotational spring stiffness.

When one considers combination of the classical boundary conditions (F, S, C) in an isotropic square plate $(a/b=1)$, there are twenty-one combinations to give distinct sets of the identical natural frequencies and this number can be theoretically determined by use of Polya counting theory [23,24]. In this theory, the number of combinations is determined by a cyclic polynomial

$$
Z_G(z) = \frac{1}{8} \left(z^4 + 2z^3 + 3z^2 + 2z \right) \tag{1}
$$

where Z_G is the number of distinct combinations to show independent sets of different natural frequencies, *G* denotes "permutation", and z is the number of different boundary conditions considered at each edge. For example, when one considers three boundary conditions (Free (F), Simple support (S) , Clamp (C)), the number "three" is inserted in Eq.(1) as $Z_G(3) = 21$.

In this paper, three combinations of S, RS and C are considered that end up with $Z_G(3) = 21$, but the combinations only by S and C (i.e., $z=2$) were already covered [4] and remaining fifteen cases

$$
Z_G(3) - Z_G(2) = 21 - 6 = 15
$$
 (2)

are treated in numerical examples, as shown as Ex.1-15 in Fig. 2.

For an isotropic rectangular plate $(a/b \ne 1)$, a cyclic polynomial becomes

$$
Z_G(z) = \frac{1}{4} \left(z^4 + 2z^3 + z^2 \right)
$$
 (3)

and the number of combinations increases up to

$$
Z_G(3) = 36\tag{4}
$$

for the combination of S, RS and C [23,24].

2.2. Ritz method considering rotational edge springs

 A semi-analytical solution is employed here as in Refs.[4,22,23] from the method of Ritz under the classical thin plate theory. The relation between stress and strain in the plate is

$$
\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \begin{bmatrix} Q_{11} & Q_{12} & 0 \\ Q_{12} & Q_{22} & 0 \\ 0 & 0 & Q_{66} \end{bmatrix} \begin{Bmatrix} \varepsilon_x \\ \varepsilon_y \\ \gamma_{xy} \end{Bmatrix}
$$
 (5)

with the matrix elements given by

$$
Q_{11} = Q_{22} = \frac{E}{1 - v^2}
$$
, $Q_{12} = vQ_{11}$, $Q_{66} = G = \frac{E}{2(1 + v)}$ (6)

where *E* is Young's modulus, *G* is a shear modulus and ν is a Poisson's ratio. When Eq.(5) is integrated through the thickness after multiplying a thickness coordinate *z*, one gets moment resultants

$$
\begin{Bmatrix} M_{x} \\ M_{y} \\ M_{xy} \end{Bmatrix} = \begin{bmatrix} D_{11} & D_{12} & 0 \\ D_{12} & D_{22} & 0 \\ 0 & 0 & D_{66} \end{bmatrix} \begin{Bmatrix} \kappa_{x} \\ \kappa_{y} \\ \kappa_{xy} \end{Bmatrix}
$$
(7)

in terms of curvature $\{K_x, K_y, K_{xy}\}$.

 If one considers the small amplitude (linear) free vibration of plate, the deflection *w* may be written by

$$
w(x, y, t) = W(x, y) \sin \omega t \tag{8}
$$

where W is the amplitude and ω is a radian frequency of the plate. Then, the maximum strain energy due to the bending is expressed by

$$
U_{\text{max}} = \frac{1}{2} \iint_A {\{\kappa\}}^T \begin{bmatrix} D_{11} & D_{12} & 0 \\ D_{12} & D_{22} & 0 \\ 0 & 0 & D_{66} \end{bmatrix} {\{\kappa\}} dA \qquad (9)
$$

where the D_{ij} are the bending stiffnesses and $\{K\}$ is a curvature vector

$$
\{\kappa\} = \left\{ -\frac{\partial^2 W}{\partial x^2} - \frac{\partial^2 W}{\partial y^2} - 2\frac{\partial^2 W}{\partial x \partial y} \right\}^T
$$
(10)

The maximum kinetic energy is also given by

$$
T_{\text{max}} = \frac{1}{2} \rho h \omega^2 \iint_A W^2 dA \tag{11}
$$

where ρ [kg/m³] is the mass per unit volume.

For the sake of simplicity, non-dimensional quantities are introduced as

$$
\xi = \frac{2x}{a}, \eta = \frac{2y}{b} \text{ (non-dimensional coordinates)},
$$

\n
$$
\alpha = a/b \text{ (aspect ratio)}, \quad d_{ij} = D_{ij} / D
$$

\n
$$
D = \frac{Eh^3}{12(1 - v^2)} \text{ (reference stiffness)} \tag{12}
$$

\n
$$
\Omega = \omega a^2 \sqrt{\frac{\rho h}{D}} \text{ (frequency parameter)}
$$

 Next, we consider the energy stored in the elastic restraints (rotational elastic springs). The energy equation is written as

$$
U_r = \frac{1}{2} \times \left\{ \int_{-b/2}^{b/2} k_{r1} \left[\frac{\partial W(-a/2, y)}{\partial x} \right]^2 dy + \int_{-a/2}^{a/2} k_{r2} \left[\frac{\partial W(x, -b/2)}{\partial y} \right]^2 dx \right\}
$$

$$
+ \int_{-b/2}^{b/2} k_{r3} \left[\frac{\partial W(a/2, y)}{\partial x} \right]^2 dy + \int_{-a/2}^{a/2} k_{r4} \left[\frac{\partial W(x, b/2)}{\partial y} \right]^2 dx \right\}
$$
(13)

where k_{ri} (i=1,2,3,4) are stiffness of rotational springs in unit $[Nm/m]=[N]$ per unit edge length. This energy is added to the plate bending energy (9).

The next step in the Ritz method is to assume the amplitude as

$$
W(\xi, \eta) = \sum_{m=0}^{M-1} \sum_{n=0}^{N-1} A_{mn} X_m(\xi) Y_n(\eta)
$$
 (14)

where A_{mn} are unknown coefficients, and $X_m(\xi)$ and $Y_n(\eta)$ are the functions modified so that any kinematical boundary conditions are satisfied at the edges [4,22,23].

 After substituting Eq.(14) into these energies, the stationary value is obtained by

$$
\frac{\partial}{\partial A_{\min}} \left\{ T_{\max} - \left(U_{\max} + U_{r,\max} \right) \right\} = 0
$$
\n
$$
\left(\overline{m} = 0, 1, 2, \dots (M-1); \overline{n} = 0, 1, 2, \dots (N-1) \right) \tag{15}
$$

Then the eigenvalue equation that contains a frequency parameter Ω is derived as

$$
\sum_{m=0}^{M-1} \sum_{n=0}^{N-1} \left[d_{11} I^{(2200)} + \alpha^2 d_{12} (I^{(2002)} + I^{(0220)}) + \alpha^4 d_{22} I^{(0022)}
$$

+4 $\alpha^2 d_{66} I^{(111)} + (\text{Spring term}) - \left(\frac{\Omega^2}{16} \right) I^{(0000)} \Big]_{m \overline{m} n} \cdot A_{mn}$
= 0 $\left(\overline{m} = 0, 1, 2, \dots (M-1); \overline{n} = 0, 1, 2, \dots (N-1) \right)$ (16)

where an integral *I* is the products

$$
I_{m\overline{m}\overline{n}\overline{n}}^{(pqrs)} = \varphi_{m\overline{n}}^{(pq)} \cdot \varphi_{n\overline{n}}^{(rs)}
$$
(17)

of the two integrals defined by

$$
\phi_{m\overline{m}}^{(pq)} = \int_{-1}^{1} \frac{\partial^{(p)} X_m}{\partial \xi^{(p)}} \frac{\partial^{(q)} X_m}{\partial \xi^{(q)}} d\xi \tag{18}
$$

and (Spring term) is the line integral along an edge.

(spring term) =
$$
\frac{1}{2}
$$
 ×
\n
$$
\left[k_{r1}^{*} \frac{dX_{m}(-1)}{d\xi} \int_{-1}^{1} Y_{n}(\eta) d\eta + k_{r2}^{*} \frac{dY_{n}(-1)}{d\eta} \int_{-1}^{1} X_{m}(\xi) d\xi + k_{r3}^{*} \frac{dX_{m}(1)}{d\xi} \int_{-1}^{1} Y_{n}(\eta) d\eta + k_{r4}^{*} \frac{dY_{n}(1)}{d\eta} \int_{-1}^{1} X_{m}(\xi) d\xi \right]
$$
\n(19)

with nondimensional rotational constant

$$
k_{r1}^* = \frac{k_{r1}a}{D}, k_{r2}^* = \frac{k_{r2}a}{D}, k_{r3}^* = \frac{k_{r3}a}{D}, k_{r4}^* = \frac{k_{r4}a}{D} \tag{20}
$$

Equation (16) is a set of linear simultaneous equations in terms of the coefficients A_{mn} , and the eigenvalues $Ω$ may be extracted by using existing computer subroutines.

The present approach uses simple polynomials

$$
X_{m}(\xi) = \xi^{m} (1 + \xi)^{\text{bc1}} (1 - \xi)^{\text{bc3}}
$$

$$
Y_{n}(\eta) = \eta^{n} (1 + \eta)^{\text{bc2}} (1 - \eta)^{\text{bc4}}
$$
(21)

 $(bc1=bc2=bc3=bc4=1)$ to represent a simply supported plate as a base plate, and the integrals (18)(19) can be exactly calculated.

2.3. Finite element formulation of rotational spring

A finite element is newly developed to include the effect of rotational springs distributed along the edges, and the finite element code (FEM code) is developed by the author to compare the result with the Ritz solution to establish accuracy of both methods. Formulation of plate bending element and kinetic element are already explained in Refs.[22,25]. Here only formulation of the edge spring element is shown.

The amplitude inside the element including boundary is assumed by

$$
W(x, y) = \{P\} \{\alpha\} \tag{22}
$$

where $\{P\}$ and $\{\alpha\}$ are (T: transpose)

$$
\{P\} = \{1, x, y, x^2, xy, y^2, x^3, x^2y, xy^2, y^3, x^3y, xy^3\}
$$
 (23)

$$
\{\alpha\} = \{\alpha_1, \alpha_2, \alpha_3, \alpha_4, \alpha_5, \alpha_6, \alpha_7, \alpha_8, \alpha_9, \alpha_{10}, \alpha_{11}, \alpha_{12}\}\
$$
\n(24)

The displacement at node *i* is defined as

$$
\{\delta_i\} = \left\{ W_i, \left(\frac{\partial W}{\partial x}\right)_i, \left(\frac{\partial W}{\partial y}\right)_i \right\}^T \tag{25}
$$

and the displacements of four nodes labelled as *i,j,k* and *l* in a rectangular element can be expressed as

$$
\left\{\delta_e\right\} = \left\{\delta_i \quad \delta_j \quad \delta_k \quad \delta_l\right\}^T
$$
 (26)

Using [*C*] which is obtained by substituting Eq.(26) into the four sets of node coordinates, *W* is transformed as

$$
W(x, y) = {P} [C]^{-1} {\delta_e}
$$
 (27)

For example, when the rotational spring is distributed uniformly along Edge(2) or Edge(4) at $y = y$, equation (27) is substituted into the second or fourth term of Eq.(13) and

$$
\frac{1}{2} \int_{-a/2}^{a/2} k_{ri} \left(\frac{\partial W(x, \overline{y})}{\partial y} \right)^2 dx = \frac{1}{2} \{ \delta_e \}^T \left[K_{ri} \right] \{ \delta_e \}
$$
\n(28)

 $(i=2,4)$ is obtained, where $[K_{ri}]$ is the *i*-th finite element of rotational edge spring

$$
\left[K_{ri}\right] = k_{ri} \left[C^{-1}\right]^{T} \left(\int \left\{\frac{\partial P(x,\overline{y})}{\partial y}\right\}^{T} \left\{\frac{\partial P(x,\overline{y})}{\partial y}\right\} dx\right) \left[C^{-1}\right]
$$
\n(29)

with $P(x, \overline{y})$ being a function of *x* at fixed $\overline{y} = -b/2$ or \overline{y} = *b* / 2 for Edge(2) and Edge (4), respectively. Spring finite elements along Edge(1) and Edge(3) can be formulated in the same manner.

3. Numerical examples and discussions

3.1. Convergence and comparison of the solution

It is assumed in numerical examples that the material is isotropic with Poisson's ratio ν=0.3. Young's modulus *E* and Poisson's ratio ν are included in the dimensionless frequency parameters $Ω$ in Eq.(12).

Figure 2 illustrates numerical examples Ex.1-Ex.15 with different degree of elastic constraints by rotational springs, and such edge (i.e., simply supported edge with uniform rotational spring attached) is denoted by "RS" (Rotational Spring). In case that the example plates have plural rotational springs on the edges, it is assumed that all the springs have the same degree of constraint.

Table 1 Convergence of (a) Ritz solution and (b)FEM solution for square plates constrained by rotational spring (Ex.1)

	Ω_1	Ω ₂	Ω_3	Ω_4	Ω_5
(a) Present Ritz solution					
$k_{\rm t}$ * = 100					
6×6	23.37	51.44	57.75	85.28	101.5
8×8	23.37	51.44	57.74	85.27	100.1
10×10	23.37	51.44	57.74	85.27	100.1
$k_+^* = 10000$					
6×6	23.64	51.67	58.65	86.14	101.7
8×8	23.64	51.67	58.64	86.13	100.3
10×10	23.64	51.67	58.64	86.13	100.3
(b) Present FEM solution					
$k_+^* = 100$					
10×10	23.24	50.98	57.31	83.61	99.16
15×15	23.31	51.21	57.53	84.47	99.61
20×20	23.33	51.31	57.62	84.81	99.80
$k_t^* = 10000$					
10×10	23.51	51.26	58.21	84.63	99.55
15×15	23.58	51.45	58.42	85.32	99.83
20×20	23.62	51.54	58.51	85.65	100.0

Table 2 Comparison of frequency parameters for square plates (Ex.15)

$$
k_r^* = k_{r1}\left(\frac{a}{D}\right) = k_{r2}\left(\frac{a}{D}\right) = k_{r3}\left(\frac{a}{D}\right) = k_{r4}\left(\frac{a}{D}\right) \tag{30}
$$

in the calculation, although they can take any different values as needed.

In the figure, Ex.1-6 are square plates constrained on $Edge(1)$ by one rotational spring, and have different combinations of RS-S-S-S, RS-C-S-S, RS-S-C-S, RS-C-C-S, RS-C-S-C and RS-C-C-C, respectively, on the remaining three edges of Edge(2)-Edge(4). Similarly, Ex.7-12 are plates constrained on two edges constrained by two rotational springs and have different combinations

	$(Ex.1, RS-S-S. \vee=0.3)$								(Ex.3 , RS
kr^*		Ω_1	Ω ₂	Ω_3	Ω_4	Ω_5		kr^*	
(0)	Ritz	19.74	49.35	49.35	78.96	98.70		(0)	Ritz
$S-S-S-S$	FEM	19.71	49.24	49.24	78.54	98.47		$S-S-C-S$	FEM
1	Ritz	20.18	49.53	50.09	79.43	98.79		1	Ritz
	FEM	20.16	49.43	49.99	79.01	98.56			FEM
10	Ritz	21.95	50.43	53.74	81.95	99.31		10	Ritz
	FEM	21.92	50.32	53.63	81.53	99.07			FEM
100	Ritz	23.37	51.44	57.74	85.27	100.1		100	Ritz
	FEM	23.33	51.31	57.62	84.81	99.80			FEM
10000	Ritz	23.64	51.67	58.64	86.13	100.3		10000	Ritz
	FEM	23.62	51.54	58.51	85.65	100.0			FEM
(infinity)	Ritz	23.65	51.67	58.65	86.13	100.3		(infinity)	Ritz
$C-S-S-S$	FEM	23.61	51.53	58.52	85.63	99.99		$C-S-C-S$	FEM

Table 3 Frequency parameters Ω of square plates

Table 4 Frequency parameters Ω of square plates

Table 5 Frequency parameters Ω of square plates

$(\mathbf{Ex.1}, \mathbf{RS-S-S.}, \nu=0.3)$						$(Ex.3, RS-S-C-S, v=0.3)$							
kr^*		Ω_1	Ω ₂	Ω_3	Ω_4	Ω_5	kr^*		Ω_1	Ω ₂	Ω_3	Ω_4	Ω_5
(0)	Ritz	19.74	49.35	49.35	78.96	98.70	(0)	Ritz	23.65	51.67	58.65	86.13	100.3
$S-S-S-S$	FEM	19.71	49.24	49.24	78.54	98.47	$S-S-C-S$	FEM	23.61	51.53	58.52	85.63	99.99
1	Ritz	20.18	49.53	50.09	79.43	98.79		Ritz	24.21	51.91	59.44	86.67	100.4
	FEM	20.16	49.43	49.99	79.01	98.56		FEM	24.18	51.77	59.31	86.16	100.1
10	Ritz	21.95	50.43	53.74	81.95	99.31	10	Ritz	26.56	53.08	63.48	89.57	101.0
	FEM	21.92	50.32	53.63	81.53	99.07		FEM	26.51	52.93	63.35	89.06	100.7
100	Ritz	23.37	51.44	57.74	85.27	100.1	100	Ritz	28.55	54.42	68.22	93.54	102.0
	FEM	23.33	51.31	57.62	84.81	99.80		FEM	28.50	54.25	68.07	92.98	101.6
10000	Ritz	23.64	51.67	58.64	86.13	100.3	10000	Ritz	28.95	54.74	69.32	94.57	102.2
	FEM	23.62	51.54	58.51	85.65	100.0		FEM	28.89	54.57	69.16	94.00	101.9 102.2 101.9
(infinity)	Ritz	23.65	51.67	58.65	86.13	100.3	(infinity)	Ritz	28.95	54.74	69.33	94.59	
C-S-S-S	FEM	23.61	51.53	58.52	85.63	99.99	$C-S-C-S$	FEM	28.89	54.55	69.17	93.96	

Figure 5 Variation of frequency parameters of square plate with spring stiffness (**Ex.3**).

Table 6 Frequency parameters Ω of square plates

	$(Ex.2, RS-C-S-S, v=0.3)$						(Ex.4 , RS-C-C-S, $v = 0.3$)							
kr^*		Ω_1	Ω ₂	Ω_3	Ω_4	Ω_{5}		kr^*		Ω_1	Ω_{2}	Ω_3	Ω_4	Ω 5
(0)	Ritz	23.65	51.67	58.65	86.13	100.3		(0)	Ritz	27.05	60.54	60.79	92.84	114.6
S-C-S-S	FEM	23.61	51.53	58.52	85.63	99.99		$S-C-C-S$	FEM	27.00	60.37	60.62	92.23	114.2
1	Ritz	24.02	52.38	58.80	86.57	101.1			Ritz	27.55	60.84	61.45	93.33	114.7
	FEM	23.98	52.24	58.67	86.06	100.8		FEM	27.50	60.68	61.28	92.72	114.4	
10	Ritz	25.54	55.89	59.57	88.89	105.9		10	Ritz	29.63	61.86	65.35	96.04	115.3
	FEM	25.49	55.75	59.43	88.38	105.6			FEM	29.57	61.69	65.19	95.43	115.0
100	Ritz	26.80	59.75	60.47	92.02	112.8		100	Ritz	31.46	63.04	69.98	99.79	116.1
	FEM	26.75	59.60	60.32	91.46	112.5			FEM	31.39	62.85	69.80	99.13	115.8
10000	Ritz	27.05	60.53	60.78	92.83	114.5		10000	Ritz	31.82	63.33	71.06	100.8	116.4
	27.00 60.38 60.63 92.26 114.2 FEM			FEM	31.76	63.13	70.88	100.1	116.0					
(infinity)	Ritz	27.05	60.54	60.79	92.84	114.6		(infinity)	Ritz	31.83	63.33	71.08	100.8	116.4
C-C-S-S	FEM	27.00	60.37	60.62	92.23	114.2		$C-C-C-S$	FEM	31.75	63.11	70.88	100.1	116.0

Figure 6 Variation of frequency parameters of square plate with spring stiffness (**Ex.4**).

(Ex.5 , RS-C-S-C, $V = 0.3$)									
$k r^\ast$		Ω_1	Ω ₂	Ω_3	Ω_4	Ω 5			
(0)	Ritz	28.95	54.74	69.33	94.59	102.2			
$S-C-S-C$	FEM	28.89	54.55	69.17	93.96	101.9			
1	Ritz	29.26	55.41	69.46	94.98	103.0			
	FEM	29.20	55.23	69.30	94.36	102.7			
10	Ritz	30.52	58.74	70.12	97.11	107.8			
	FEM	30.46	58.56	69.95	96.49	107.4			
100	Ritz	31.61	62.47	70.89	100.0	114.5			
	FEM	31.54	62.27	70.70	99.35	114.2			
10000	Ritz	31.82	63.32	71.07	100.8	116.3			
	FEM	31.75	63.12	70.89	100.1	116.0			
(infinity)	Ritz	31.83	63.33	71.08	100.8	116.4			
C-C-S-C	FEM	31.75	63.11	70.88	100.1	116.0			

Table 7 Frequency parameters Ω of square plates

Table 8 Frequency parameters Ω of square plates

Table 9 Frequency parameters Ω of square plates

	$(Ex.7, RS-RS-S, v=0.3)$									
kr^*				Ω_1 Ω_2 Ω_3 Ω_4		Ω_{5}				
$S-S-S-S$	Ritz			19.74 49.35 49.35 78.96 98.70						
1	Ritz			20.62 50.27 50.27 79.89		99.64				
10	Ritz			23.96 54.67 54.79 84.83		105 0				
100	Ritz			26.55 59.44 59.68 91.19		112.5				
10000	Ritz			27.05 60.53 60.78 92.82		114.5				
$C-C-S-S$	Ritz			27.05 60.54 60.79 92.84		114 6				

Table 10 Frequency parameters Ω of square plates

Table 11 Frequency parameters Ω of square plates

Table 12 Frequency parameters Ω of square plates

Table 13 Frequency parameters Ω of square plates

of simple support (without spring) and clamped edges. Ex.13 and 14 have three edges with rotational springs, and Ex.15 is a simply supported plate with all edges constrained by rotational springs.

Convergence study is presented in Table 1 for frequency parameters in Ex.1 obtained by the present (a) Ritz method and (b) finite element method. In both sets of results, nondimensional spring constants are assumed as *k*r*=100 and 10000. In (a), the number of series terms in Eq.(14) are taken as $M \times N = 6 \times 6$, 8×8 and 10×10 , and very fast convergence from above (i.e., this solution is upperbounded) is observed within the four significant figures. In (b), the number of finite elements is taken as 10×10 , 15×15 and 20×20, and slightly slower convergence from below is observed as compared to the Ritz solution. This nonconforming finite element solution seems to give lower bound in this problem, but no theoretical proof is possible due to use of this non-conforming element. The discrepancy between the Ritz 10×10 solution and FEM 20×20 solution is 0.60 percent in the maximum and 0.29 percent on the average. Generally both different solutions agree well.

Table 2 presents a comparison in Ex.15 (uniformly constrained on the four edges) between Refs.[16,17] by Li and co-workers and the present Ritz result. The present solution here is given in the five significant figure to match to the result [17], and clearly they show excellent agreement. Thus in both Tables 1 and 2, validity of the present two methods is well established.

3.2. Frequency parameters of square plates

 Tables 3-8 present pairs of frequency parameters obtained by the two different methods for the lowest five modes of square plates (*a/b*=1) in Ex.1-Ex.6, respectively. The degree of rotational springs is increased as $k_r^* = 0$ (totally simply supported edge), 1, 10, 100, 10000 (almost clamped). In the limiting case of $k_r^* = \infty$ (infinity), the accurate values are available by replacing $k_r^* = \infty$ (S) with clamped edge (C). It is seen in common that the frequencies are monotonically increasing, as RS edge starts from simple support $(k_r^* = 0)$ to strongly constrained edge k_r^* =10000=10⁴, and this degree of k_r^* =10⁴ virtually coincide with the clamped edge.

Such monotonical increases in frequency can be seen in the accompanying Figs.3-8 for Ex-1-Ex.6, respectively. Some interesting observations are made in each figure. For example, in Fig.3 (Table 3), the second and third frequencies of S-S-S-S plate are identical (degenerated mode of a square plate) for $k_r^* = 0$, but they become separated as k_{r}^{*} being increased and become the second and third frequencies of C-S-S-S plate. In contrast in Fig.8 (Table 8), two distinct second and third frequencies of S-C-C-C plate gradually approach each other and eventually merge into one degenerated mode.

Tables 9-17 list the lowest five frequencies obtained by the Ritz method only, when needed, they can be plotted in figures by using Excel and other software. When one needs frequency value for intermediate stiffness, one can introduce interpolation curves with respect to the stiffness values of k_r^* = 0, 1, 10, 100, 10000 and ∞.

Table 14 Frequency parameters Ω of square plates

$(Ex.12, RS-C-RS-C, v=0.3)$

kr^*			Ω_1 Ω_2 Ω_3 Ω_4 Ω_5	
S-C-S-C Ritz 28.95 54.74 69.33 94.59 102.2				
1	Ritz 29.57 56.08 69.59 95.37 103.9			
10	Ritz 32.42 62.91 71.01 99.80 113.3			
100	Ritz 35.33 71.29 72.89 106.3 127.7			
10000	Ritz 35.98 73.37 73.39 108.2 131.6			
C-C-C-C Ritz 35.99 73.39 73.39 108.2 131.6				

Table 15 Frequency parameters Ω of square plates

$S-C-S-C$		Ω_1	Ω_2	Ω_3	Ω_4	Ω_5			
	Ritz	28.95	54.74	69.33	94.59	102.2			
1	Ritz	29.57	56.08	69.59	95.37	103.9			
10	Ritz	32.42	62.91	71.01	99.80	113.3			
100	Ritz	35.33	71.29	72.89	106.3	127.7			
10000	Ritz	35.98	73.37	73.39	108.2	131.6			
$C-C-C-C$	Ritz	35.99	73.39	73.39	108.2	131.6			
Table 15 Frequency parameters Ω of square plates $(Ex.13, RS-RS-RS-S, v=0.3)$									
kr^*		Ω_1	Ω_2	Ω_3	Ω_4	Ω_5			
$S-S-S-S$	Ritz	19.74	49.35	49.35	78.96	98.70			
$1\,$	Ritz	21.07	50.46	51.01	80.36	99.74			
10	Ritz	26.33	55.86	59.17	87.87	105.6			
100	Ritz	30.88	61.90	68.73	98.0	114.1			
10000	Ritz	31.81	63.31	71.05	100.8	116.3			
$C-C-C-S$	Ritz	31.83	63.33	71.08	100.8	116.4			
Table 16 Frequency parameters Ω of square plates $(Ex.14, RS-RS-RS-C, v=0.3)$									
kr^*		Ω_1	Ω_2	Ω_3	Ω_4	Ω_5			
$S-S-S-C$	Ritz	23.65	51.67	58.65	86.13	100.3			
1	Ritz	24.95	53.32	59.75	87.52	102.1			
10	Ritz	30.29	61.45	65.30	95.04	112.2			
100	Ritz	35.00	71.03	71.82	105.4	127.5			
10000	Ritz	35.98	73.37	73.37	108.2	131.5			
$C-C-C-C$	Ritz	35.99	73.39	73.39	108.2	131.6			
Table 17 Frequency parameters Ω of square plates $(Ex.15, RS-RS-RS-RS, v=0.3)$									
kr^*		Ω_1	Ω_2	Ω_3	Ω_4	Ω_5			
$S-S-S-S$	Ritz	19.74	49.35	49.35	78.96	98.70			
1	Ritz	21.50	51.19	51.19	80.83	100.6			
10	Ritz	28.50	60.22	60.22	90.81	111.2			
100	Ritz	34.67	70.78	70.78	104.5	127.0			
10000	Ritz	35.97	73.36	73.36	108.2	131.5			
$C-C-C-C$	Ritz	35.99	73.39	73.39	108.2	131.6			

Table 16 Frequency parameters Ω of square plates

(**Ex.14**, RS-RS-RS-C, ν=0.3)

kr^*		Ω_1 Ω_2 Ω_3 Ω_4 Ω_5	
S-S-S-C Ritz 23.65 51.67 58.65 86.13 100.3			
1	Ritz 24.95 53.32 59.75 87.52 102.1		
10	Ritz 30.29 61.45 65.30 95.04 112.2		
100	Ritz 35.00 71.03 71.82 105.4 127.5		
10000 Ritz 35.98 73.37 73.37 108.2 131.5			
C-C-C-C Ritz 35.99 73.39 73.39 108.2 131.6			

Table 17 Frequency parameters Ω of square plates

3.3. Frequency parameters of rectangular plates

Table 18 Frequency parameters Ω of rectangular plate Table 19 Frequency parameters Ω of rectangular plate

 $(a/b=2/3, v=0.3)$

 $(a/b=1.5, v=0.3)$

Figure 9 Mode shapes (nodal lines) of square plates (**Ex.2**) (〇: Maximum amplitude)

by rotational springs), Ex.13 (three edges) and Ex.15 (four edges), where all examples here start from S-S-S-S plate. Since the present frequency parameter uses the side length *a* along *x* axis (Fig.1) in Eq.(12), a rectangular plate $(a/b=2/3)$ appears to have more area than a plate $(a/b=1.5)$, and therefore frequency values of *a/b*=2/3 in Table 18 have lower values than those of the smaller plate of *a/b*=1.5. The frequency values for other aspect ratios may be approximated by three-point interpolation or extrapolation curves using values for *a/b*=2/3, 1 and 1.5 in the paper.

3.4. Vibration mode of square plates

In free vibration analysis, vibration mode shapes are also important technical information. Nodal lines (line of zero amplitude) are plotted in Fig.9, as one example, for a square plate with one rotational spring on $Edge(1)$ (Ex.2 in Fig.2). Variations of nodal lines are illustrated starting from *k*r*=0 (S-C-S-S plate) to *k*r*=∞ (C-C-S-S plate) by increasing rotational spring stiffness as *k*r*=10 and 1000. The maximum amplitude and nodal line are given as a circle \bigcirc and thick solid lines in each figure.

All the first modes are $(m^*, n^*) = (1,1)$ mode, where m^* and *n** are half wave numbers to describe modal shapes. There appear no nodal lines in the fundamental modes. The second and third modes are $(2,1)$ and $(1,2)$ modes, respectively. As the stiffness *k*r* is increased, nodal lines

start skewed, and eventually these two modes merge and degeneration of $(2,1)$ and $(1,2)$ mode occurs to show the same nodal pattern. The fourth mode is (2,2) mode and nodal lines are kept almost straight and do not deform much. The fifth mode is (3,1) mode and nodal lines are skewed due to the stiffness increase. A nodal circle is formed by superposing (3,1) and (1,3) mode, as observed for totally clamped (C-C-C-C) square plate [1].

4. Conclusions

The present paper has illustrated a straightforward application of Ritz method to accurately determine the natural frequencies of a rectangular plate with uniform rotational elastic springs located on any of the four edges. A set of additional energy terms due to the springs was added to the plate strain energy in bending. The effects of the rotational springs on determining frequencies of the simply supported rectangular plates were comprehensively investigated through numerical results, including careful convergence and comparison studies. Accurate frequencies were tabulated for all the possible combinations of rotationally constrained edge(s) and other simply supported or clamped edges. It is expected that the comprehensive data is useful for researchers and design engineers.

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